

Subject code: 17421

MAHARASHTRA STATE BOARD OF TECHNICAL EDUCATION (Autonomous) (ISO/IEC-270001 – 2005 certified)

WINTER-2014 EXAMINATION

Model Answer

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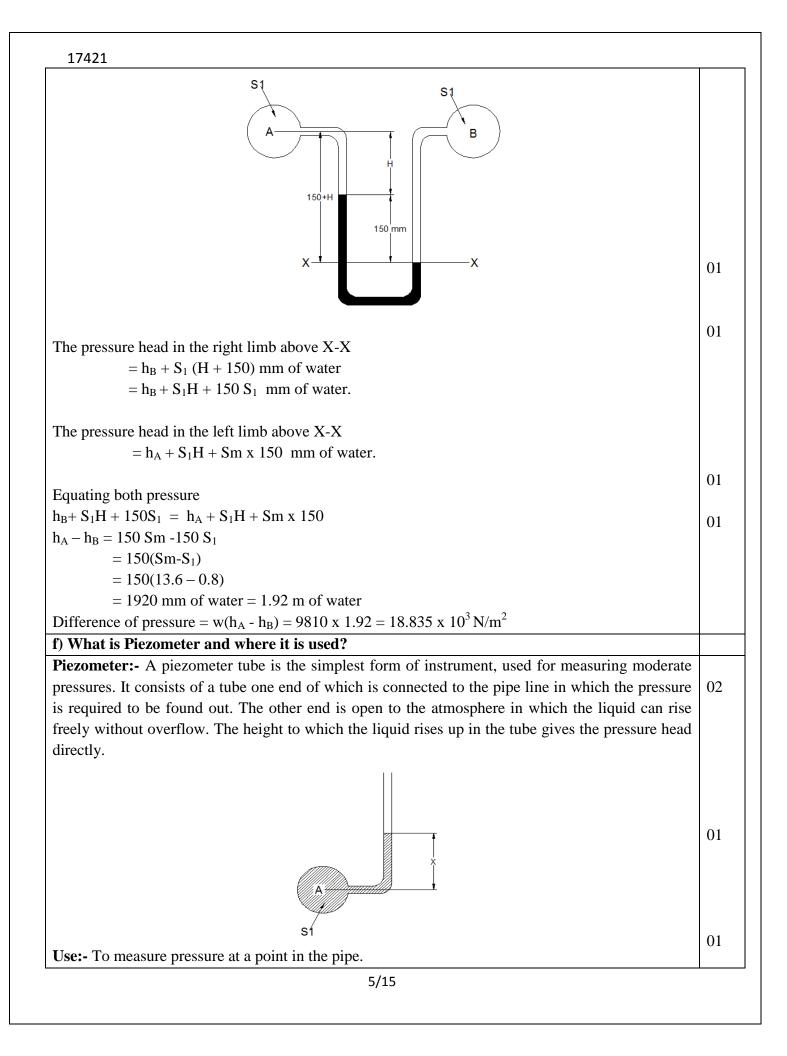
Important Instructions to examiners:

- 1) The answer should be examined by keywords and not as word-to-word as given in the model answer scheme.
- 2) The model answer and the answer written by candidate may vary but the examiner may try to assess the understanding level of the candidate.
- 3) The language error such as grammatical, spelling errors should not be given more importance.(Not applicable for subject English and communication skill).
- 4) While assessing figures, examiner may give credit for principal components indicated in the figure. The figure drawn by candidate and model answer may vary. The examiner may give credit for any equivalent figure drawn.
- 5) Credits may be given step wise for numerical problems. In the some cases, the assumed constants values may vary and there may be some difference in the candidates answer and model answer.
- 6) In case of some questions credit may be given by judgment on part of examiner of relevant answer based on candidates understanding.

Q1 Attempt any TEN of the following	20
a) Define mass density & state its SI unit.	<u> </u>
Mass Density:- The mass density of a liquid may be defined as the mass per unit volume at	01
standard temperature.	
SI unit :- kg/m ³	01
b) State four application of Hydraulics in environmental engineering.	
Ans:-Four applications of Hydraulics in environmental engineering.	*
i) To design water distribution system from reservoir.	
ii) To determine the pressure head of water supply system.	
iii) To determine the power required for pumps.	
iv) To measure the pressure at a point.	
v) To design the pipe diameter of water supply line as well as sewer system.	
*(Any four Points each 1 Mark each)	
c) Define ideal fluid and real fluid.	
Ans:- Ideal Fluid:-A fluid, which does not possess viscosity, Surface tension and compressibility	01
is known as ideal fluid.	
Real Fluid:- A fluid, which possess viscosity, Surface tension and compressibility is known as real	01
fluid.	

17421 d) State Newton's law of viscosity.	
Ans:- It states that "The shear stress on a layer of a fluid is directly proportional to the rate of shear	
strain".	02
$\varsigma = (v/y) = \mu x (v/y)$	
e) A pressure of 1.2 Pascal applied to 650 litres of liquid caused a volume reduction by 1.5	
litres. Calculate bulk modulus of elasticity for liquid	
Ans:-	
Given Data:- $d_p = 1.2$ pa. = 1.2 N/m ² , V = 650 litres, $d_v = 1.5$ litres	
Bulk Modulus = $K = -(dP/(dv/V))$	01
= -(1.2/(1.5/650))	
$= -520 \text{ N/m}^2$	01
f) Define pressure &. State its SI unit.	
Ans:- Pressure at a point due to a liquid is defined as the force acting per unit area.	01
SI unit :- N/m ²	01
g) How will you measure negative pressure?	
Ans:- Mercury in the U-tube is deflected by h_2 due pressure at point A, as deflection occurs in the	
left limb indicates that, pressure at A is negative (vacuum), pressure above the horizontal datum x-x	
in the left and right limb of the manometer should be same	
	01
Pressure above x-x in the left limb = $S_1h_2 + S_1h_1 + P_A$	
Pressure above x-x in the right limb = 0	01
$P_A = -(S_1h_2 + S_1h_1)$	01
h) Define Reynolds number Ans:- The Reynolds number is the ratio of inertia force to the viscous force.	01
$R_N = $ (Inertia Force/ Viscous Force)	01
K _N – (incitia Force/ viscous Force)	01
i) Mention necessity of inverted manometer	
Ans:-1) To measure low and negative pressure differences in two pipes.	01
2) To measure sensitive pressure	01
j) Write modified Darcy-Weisbach equation	
Ans:- Modified Darcy-Weisbach Equation	02
$hf = \frac{fLQ^2}{12.1 D^5}$	
k) Define Froude's number	
	-
·	1
Ans:- The Froude's number is defined as the square root of the ratio of inertia force of flowing fluid to	02

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$Fe = \sqrt{\frac{Fi}{Fg}}$		
l) List four uses	s of pitot tube	
Ans:-		
i) The pitot tube	e is a simple and convenient instrument to measure the difference between static,	1⁄2
total and dyn	namic pressure (or head)	(each)
ii) To measure t	he velocity in cannel	
	the velocity at a point in flow stream	
	tube can measure the fluid flow velocity by converting the kinetic energy in the	
fluid flow in	to potential energy	
Q2. Attempt a	ny FOUR of the following	16
a) State Pascal'	's law & its practical application	
Ans:-		
Pascal's Law: -	• Pascal's Law states that the pressure or intensity of pressure at a	02
	point in a static fluid is equal in all directions	
Application: -	Law is applied in the construction of machines used for multiplying forces e.g.	02
	hydraulic jack, hydraulic pressure, hydraulic lift, hydraulic crane, hydraulic	
	river, etc.	
pressure.	e surface Determine the total pressure on the plate and depth of centre of	
	e surface Determine the total pressure on the plate and depth of centre of	
pressure.Ans:_Given Data :- $d = 1.5 m$ $x = 2.5 m$	e surface Determine the total pressure on the plate and depth of centre of $\overline{2.5}$ 1.75 1.75 1.75 1.5 m Dia	
pressure. Ans:_ Given Data :- d = 1.5 m x = 2.5 m Sketch	₹ 1.75 2.5 0.75 c.e.	
pressure. Ans:_ Given Data :- d = 1.5 m x = 2.5 m Sketch Solution :-	$ \begin{array}{c} \hline \\ \hline $	
pressure. Ans:_ Given Data :- d = 1.5 m x = 2.5 m Sketch Solution :- Area = $A = (\pi/4)$	$\frac{1}{2.5}$	
pressure. Ans:_ Given Data :- d = 1.5 m x = 2.5 m Sketch Sketch Solution :- Area = A = $(\pi/4)$ = $(\pi/4)$	$\frac{1.75}{1.75}$	
pressure. Ans:_ Given Data :- d = 1.5 m x = 2.5 m Sketch Sketch Solution :- Area = A = $(\pi/4)$ = $(\pi/4)$ i) Total	$\frac{1.75}{1.5 \text{ m Dia}}$	01
pressure. Ans:_ Given Data :- d = 1.5 m x = 2.5 m Sketch Sketch Solution :- Area = A = $(\pi/4)$ = (π) i) Total F= w	$\frac{1.75}{1.5 \text{ m Dia}}$	

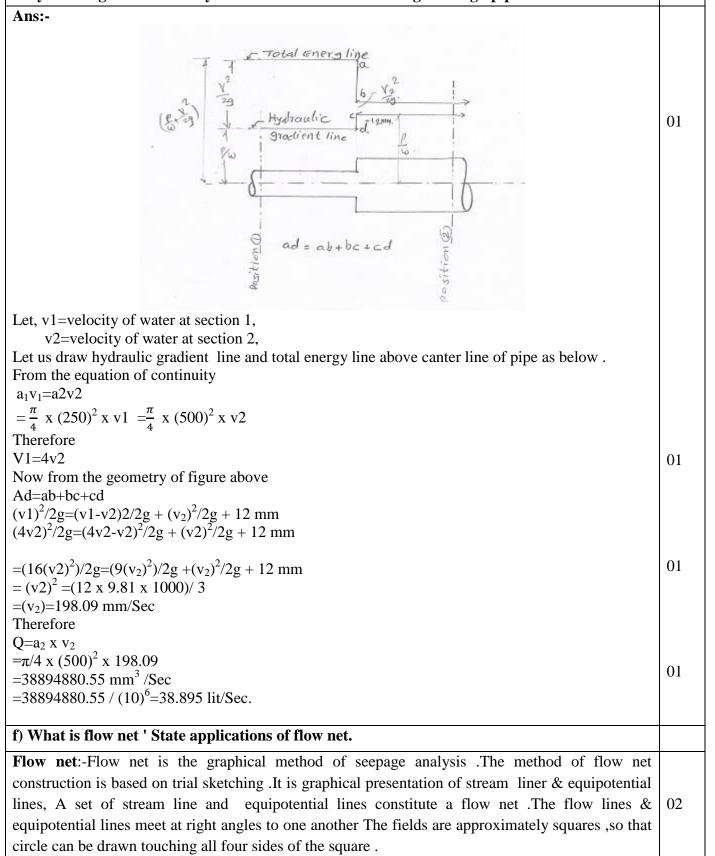


Q3. Attempt any FOUR of the following	16
a) Explain with neat sketch the working of Burdon's tube pressure guage	
Ans:-	
Diagram :	
Scale	
Bourdon	
tube	
	02
Pinion Adjustable Link	
Sector_	
Pivot Segment lever	
Stop	
Socket	
www.InstrumentationToday.com	
Bourdon Tube Pressure Gauge	
Working:-Bourdon tube pressure gauge is used to measure high pressure. It consists of tube as	
shown in fig. having elliptical cross section. This tube is called as Bourdons Tube. One end of this	
tube is connected the point whose pressure is to be measured and other end free. When fluid enters	
in the tube elliptical cross section of tube becomes circular. Due to this the free end of tube shifts	
outward. This motion is transferred through link and pointer arrangement. The pointer moves over	02
a calibrated scale, which directly indicates the pressure in terms of N/m^2 or m head of mercury.	02
b) State the practical applications and limitations of Bernoulli's theorem	
Ans:-	
Practical applications of Bernoulli's theorem:-	02
1) In all problem of incompressible fluid flow where energy consideration are involved.	(any
2) To derived various formula in hydraulics.	two
3) To find discharge through venturimeter, to find dimension of convergent cone throat, divergent	
cone. 4) To find discharge in pipe using an orifice meter.	
5) To find the velocity and flow at the required point a pipe or stream using pitoot tube.	
limitations of Bernoulli's theorem are below :-	
Bernoulli's theorem has been derived an certain assumptions ,which are rarely possible ,which	02
show that Bernoulli's theorem , has same limitation in the application may be as .	(an
Bernoulli's theorem is not applicable :-	two
1) For fluid with non-uniform flow.	
2) For fluid with unsteady flow.	
3) For fluid with zero viscosity.	
4) For fluid flow where there is no loss of energy.	
5) For fluid flow with other than gravitational force.	

Laminar Flow	Turbulant flow	*
1) Each Pratical moves in a definite	1) The fluid partical continuously mis	
path and do not cross each other.	and cross each other.	
2) It occurs at low velocity of flow.	2) It occurs at high velocity of flow.	
3) This flow occurs in viscous fluids	3) This flow occurs in fluid having very less viscosity.	
4) Reynold's number is less than 2000	4) Reynold's number is less than 2000	
5) Fluid particle move in layers with one layer over the another.	5) Fluid Particle moves in disorderly manner, they cross the path of each other.	
6) Sketch	6) Sketch	
7) Example	7) Example	
*(Any four	Points each 1 Mark)	
are 100 kN/m2 & 60 kN/rn2 respectively an	es in diameter from 300 mm at position 1 to 600 a higher level If the pressure at position I & 2 nd the discharge is 300 lit/s. Determine the loss o	f
are 100 kN/m2 & 60 kN/rn2 respectively an head	a higher level If the pressure at position I & 2	f
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m ,	a higher level If the pressure at position I & 2	f
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m , p1=100 KN/m 2 , p2=60 KN/m 2	a higher level If the pressure at position I & 2	f
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m , p1=100 KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec	a higher level If the pressure at position I & 2	f
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m , p1=100 KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation	a higher level If the pressure at position I & 2	f
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d_1 =300mm , $d2$ =600mm , $z1$ =0 m , $z2$ =5m , p1=100 KN/m 2 , $p2$ =60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2	a higher level If the pressure at position I & 2	f
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d_1 =300mm ,d2=600mm ,z1=0 m , z2=5m , p_1 =100 KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2 $a_1 = \frac{\pi}{4} \times (0.3)^2 = 0.07068 \text{ m}^2$	a higher level If the pressure at position I & 2	
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d_1 =300mm ,d2=600mm ,z1=0 m , z2=5m , p_1 =100 KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2 $a_1 = \frac{\pi}{4} \times (0.3)^2 = 0.07068 \text{ m}^2$	a higher level If the pressure at position I & 2	f 01
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d_1 =300mm ,d2=600mm ,z1=0 m , z2=5m , p1=100 KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2 $a_1 = \frac{\pi}{4} \times (0.3)^2 = 0.07068 \text{ m}^2$ $a_2 = \frac{\pi}{4} \times (0.6)^2 = 0.28274 \text{ m}^2$	a higher level If the pressure at position I & 2	
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d_1 =300mm ,d2=600mm ,z1=0 m , z2=5m , p1=100 KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2 $a_1 = \frac{\pi}{4} \times (0.3)^2 = 0.07068 \text{ m}^2$ $a_2 = \frac{\pi}{4} \times (0.6)^2 = 0.28274 \text{ m}^2$ 0.3=0.07068 x v ¹ =0.28274 x v ²	a higher level If the pressure at position I & 2	
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m , $p_1=100$ KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2 $a_1 = \frac{\pi}{4} \times (0.3)^2=0.07068 \text{ m}^2$ $a_2 = \frac{\pi}{4} \times (0.6)^2=0.28274 \text{ m}^2$ $D.3=0.07068 \times v^1=0.28274 \times v^2$ Therefore , $v^1=4.244$ m /Sec	a higher level If the pressure at position I & 2	
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m , $p_1=100$ KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2 $a_1 = \frac{\pi}{4} \times (0.3)^2 = 0.07068 \text{ m}^2$ $a_2 = \frac{\pi}{4} \times (0.6)^2 = 0.28274 \text{ m}^2$ $0.3=0.07068 \times v^1 = 0.28274 \times v^2$ Therefore , $v^1 = 4.244 \text{ m/Sec}$ $v^2 = 1.061 \text{ m/Sec}$	a higher level If the pressure at position I & 2 nd the discharge is 300 lit/s. Determine the loss o	
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are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m , $p_1=100$ KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2 $a_1 = \frac{\pi}{4} \times (0.3)^2 = 0.07068 \text{ m}^2$ $a_2 = \frac{\pi}{4} \times (0.6)^2 = 0.28274 \text{ m}^2$ $0.3=0.07068 \times v^1 = 0.28274 \times v^2$ Therefore , $v^1 = 4.244 \text{ m/Sec}$ $v^2 = 1.061 \text{ m/Sec}$ Now, Total head at position 1 -Using bernoulli' $=Z_1 + (P_1) / \gamma + (v_1)^2 / 2g$	a higher level If the pressure at position I & 2 nd the discharge is 300 lit/s. Determine the loss o	
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m , $p_1=100 \text{ KN/m } 2$, $p_2=60 \text{ KN/m } 2$ Q=discharge =300Lit/Sec =0.3m3/Sec Jsing continuity equation Q=a1v1=a2v2 $h_1 = \frac{\pi}{4} \times (0.3)^2 = 0.07068 \text{ m}^2$ $h_2 = \frac{\pi}{4} \times (0.6)^2 = 0.28274 \text{ m}^2$ 0.3=0.07068 x v ¹ =0.28274 x v ² Therefore , V^1 =4.244 m /Sec V^2 =1.061 m/Sec Now, Total head at position 1 -Using bernoulli' =Z ₁ +(P ₁) / γ + (v ₁) ² / 2g =0+(100)/(9.81 x 0.8) + (4.244) ² /(2 x 9.981)=13	a higher level If the pressure at position I & 2 nd the discharge is 300 lit/s. Determine the loss o s equation 6.6601 m of water	01
are 100 kN/m2 & 60 kN/rn2 respectively an head Ans:-Given oil sp.gr =0.8 d ₁ =300mm ,d2=600mm ,z1=0 m , z2=5m , $p_1=100$ KN/m 2 , p2=60 KN/m 2 Q=discharge =300Lit/Sec =0.3m3/Sec Using continuity equation Q=a1v1=a2v2 $a_1 = \frac{\pi}{4} \ge (0.3)^2 = 0.07068 \text{ m}^2$ $a_2 = \frac{\pi}{4} \ge (0.6)^2 = 0.28274 \text{ m}^2$ $0.3=0.07068 \ge v^1 = 0.28274 \ge v^2$ Therefore , $v^1 = 4.244 \text{ m/Sec}$ $v^2 = 1.061 \text{ m/Sec}$ Now, Total head at position 1 -Using bernoulli' $=Z_1 + (P_1) / \gamma + (v_1)^2 / 2g$ $=0 + (100)/(9.81 \ge 0.8) + (4.244)^2/(2 \ge 9.981) = 13$ Total head at position 2 -Using bernoulli's equal	a higher level If the pressure at position I & 2 nd the discharge is 300 lit/s. Determine the loss o s equation 6.6601 m of water	01
are 100 kN/m2 & 60 kN/rn2 respectively an	a higher level If the pressure at position I & 2 nd the discharge is 300 lit/s. Determine the loss o s equation 6.6601 m of water ation	01

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e) At a sudden enlargement of water line a 250 mm diameter to 500 mm diameter pipe, the hydraulic gradient uses. by 12 mm Calculate the discharge through pipe.



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Applications of flow net:- 1) In seepage analysis of dam foundation	
2) In seepage analysis of body of dam ,	02
3) In the design of hydraulic structures.	02
Q4. Attempt any FOUR of the following.	16
a) Define friction factor & state any four factors affecting friction factor.	<u> </u>
Friction Factor :- A dimension less quantity depends upon the roughness inside the pipe ,viscosity of liquid flowing throw pipes which affect's head loss is known as Friction Factor. factors affecting friction factor:-(ANY TWO)	02
 Nature of Surface of Pipe material Pipe diameter Length of pipeline Head loss Square of Velocity of flow b) Explain HGL and TEL with curve. 	02
Hydraulic Gradient Line (HGL): It is the line which gives the sum of pressure head (p/w) and datum head (z) of a flowing fluid in a pipe with respect to some reference line .Hydraulic gradient line is obtained by joining the top of all vertical ordinates, showing the pressure head (p/w) of flowing fluid in a pipe from the center of the pipe.	
Total Energy Line(TEL):): It is the line which gives the sum of pressure head(p/w),datum head(z) and kinetic head($v^2/2g$) of a flowing fluid in a pipe with respect to some reference line .Total energy line is obtained by joining the tops of all vertical ordinates, showing the pressure head(p/w) and kinetic head($v^2/2g$) of flowing fluid in a pipe from the center of the pipe.	01
TEL	
$p/w + v^2/2g + z$ p/w + z $p/w + z$ $p/w + z$	02
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c) What do you mean by water hammer ' State its causes (any three)	
Ans:-	
HA HA Water Hammer	
Water Hammer-When the water flowing in a long pipe is suddenly brought to rest by closing the valve, there will be a sudden rise in pressure due to the momentum of the moving water being destroyed. This causes a wave of high pressure to be transmitted along the pipe which creates noise known as water hammer .The rise in pressure in some cases may be so large that the pipe may even burst. Therefore it is essential to take into account this pressure rise in the design of pipes. The magnitude of pressure rise depends on the speed at which the valve is closed, velocity of flow, length of the pipe and elastic properties of the pipe material as well as flowing fluid. Causes.	02
If a fluid flowing in a pipe is suddenly brought to rest by closing the valve, there will be sudden rise in pressure due to momentum of the moving fluid being destroyed.	02
1) This causes a series of pressure vibrations.	
2) These vibrations setups noises in the pipe.	
 d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- 	
 d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- Q=0.37m³/Sec 	
 d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- Q=0.37m³/Sec D1=.25 m ,d2=0.30 m , 	
 d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- Q=0.37m³/Sec D1=.25 m ,d2=0.30 m , A1=(π/4 x (0.25)² =0.049 m² 	01
d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- $Q=0.37m^3/Sec$ D1=.25 m, d2=0.30 m, $A1=(\pi/4 \text{ x} (0.25)^2 = 0.049 \text{ m}^2$ $A2=(\pi/4 \text{ x} (0.30)^2 = 0.0707 \text{ m}^2$	
d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- $Q=0.37m^3/Sec$ D1=.25 m, d2=0.30 m, $A1=(\pi/4 \text{ x} (0.25)^2 = 0.049 \text{ m}^2$ $A2=(\pi/4 \text{ x} (0.30)^2 = 0.0707 \text{ m}^2$ V1=(0.37/0.049)=7.5510 m/Sec	01 01
d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- $Q=0.37m^3/Sec$ D1=.25 m, d2=0.30 m, $A1=(\pi/4 \text{ x} (0.25)^2 = 0.049 \text{ m}^2$ $A2=(\pi/4 \text{ x} (0.30)^2 = 0.0707 \text{ m}^2$ V1=(0.37/0.049)=7.5510 m/Sec V2=(0.37/0.0707)=5.2333 m/Sec	
d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- $Q=0.37m^3/Sec$ D1=.25 m, d2=0.30 m, $A1=(\pi/4 \text{ x} (0.25)^2 = 0.049 \text{ m}^2$ $A2=(\pi/4 \text{ x} (0.30)^2 = 0.0707 \text{ m}^2$ V1=(0.37/0.049)=7.5510 m/Sec	01
d) A diameter of a horizontal pipe suddenly changes from 25 cm to 30 cm. Calculate the loss of head, if discharge is 370 lit/sec Ans:- $Q=0.37m^3/Sec$ D1=.25 m, d2=0.30 m, $A1=(\pi/4 x (0.25)^2 = 0.049 m^2)$ $A2=(\pi/4 x (0.30)^2 = 0.0707 m^2)$ V1=(0.37/0.049)=7.5510 m/Sec V2=(0.37/0.0707)=5.2333 m/Sec Head loss $=(v1-v2)^2/2g =(7.5510-5.2333)^2/2 x 9.81$	01 01

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Total head = frictional head $=$
$H=hf=(fLv^2)/2 x g x D$
$15=(0.005 \text{ x } 400 \text{ x } \text{ v}^2)/(2 \text{ x } 9.81 \text{ x } 0.2)$
V=2.765 m/Sec
Discharge =Q=($\pi/4 \ge (D)^2$) $\ge v$
$Q=(\pi/4 \ge (0.2)^2) \ge 2.765$
Q=0.0868 m3/Sec
Q=86.86 lit/Sec

f) Water is flowing through a rectangular channel of width 8 in bed slope 1 in 1000. Depth of flowing channel is 5 in. Find the discharge through the channel. Take Chezy's constant C = 50.

01

01 01

01

01

01

01

01 16

01

01

01

Ans:-

A=(8 x 5) =40 m² Q=(a x v) Using chezy's formula, V= $c\sqrt{mi}$ m=hydraulic mean depth = A/p i= bed slope =1 in 1000 =0.001,c= chezy's constant =50 p=(b+2d)=(8+2 x 5)=18 m m=hydraulic mean depth = A/p=(40/18)=2.22 Q=(a x $c\sqrt{mi}$)=(40 x 50 x $\sqrt{(2.22 X 0.001)}$) Q =94.2337 m³/Sec Q=94233.7 lit /Sec

Q5. Attempt any TWO of the following:

a) A trapezoidal channel section has side slope 2 vertical to 3 horizontal. It is discharging water at a rate of 20 cumecs with bed slope 1 in 2000. Design the channel for its best form. Take Manning's constant 0.01.

Ans:-

Type of channel : trapezoidal channel , Side slope of channel =3H: 2V, Discharge =Q= 20 cumecs = 20 m^3 /sec ,Bed slope = i=1 in 2000 = 1/2000 ,Mannigs constant =N=0.01

Let ,b = base width of channel , d = depth of flowSide slope 3H:2V n/1=3/2 n=3/2Trapezoidal channel is designed for best form, We have condition $d\sqrt{(n^2+1)}=1/2(b+2nd)$ $d\sqrt{[(3/2)2 + 1]}=1/2[b+2x(3/2) x d]$ 1.80 d=0.5b + 1.5d0.3d=0.5b**b =0.6 d** Put the value of b in below eqn,

A=(b+ nd) d A= $[0.6d + (3/2) \times d]d$

 $A=2.1 d^2$

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Wetted perimeter = $P = 2d \sqrt{(n^2+1)} + b$
$= 2d\sqrt{[(3/2)2 + 1]} + 0.6 d$
= 4.205 d
Hydraulic radius =R=A/P
$= 2.1 d^2/4.205 d = 0.499 d = 0.5 d$

Manning's formula $Q=Ax1/N R^{2/3} i^{\frac{1}{2}}$ $20 = 2.1d^2 (1/0.01) x (0.5 d)^{2/3} x (1/2000)^{1/2}$ **d** = depth of flow = **2.36 m**

b = base width of channel =0.6 d =0.6 x 2.36= 1.42 m

b) What do you mean by Hydraulic Jump? Explain with sketch. State the types of hydraulic jump with Froude's number.

01

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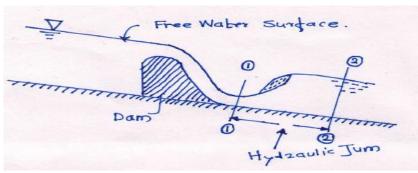
01

01

02

02

Ans:-



Hydraulic Jump:-

Consider the flow of water over a dam .The height of water at section 1-1 is small; as we move toward downstream the height or depth of water increases rapidly over a short length of the channel 04 this is because at the section 1-1 is less than critical depth. Shooting flow is an unstable type of flow is and does not continue on the downstream side. Then this shooting will convert itself into a streaming flow & hence depth of water will increase. This sudden increase of depth of water is called a hydraulic jump.

OR when supercritical flow changes to subcritical then sudden rise of water occurs, this sudden increase of depth of water is called a hydraulic jump.

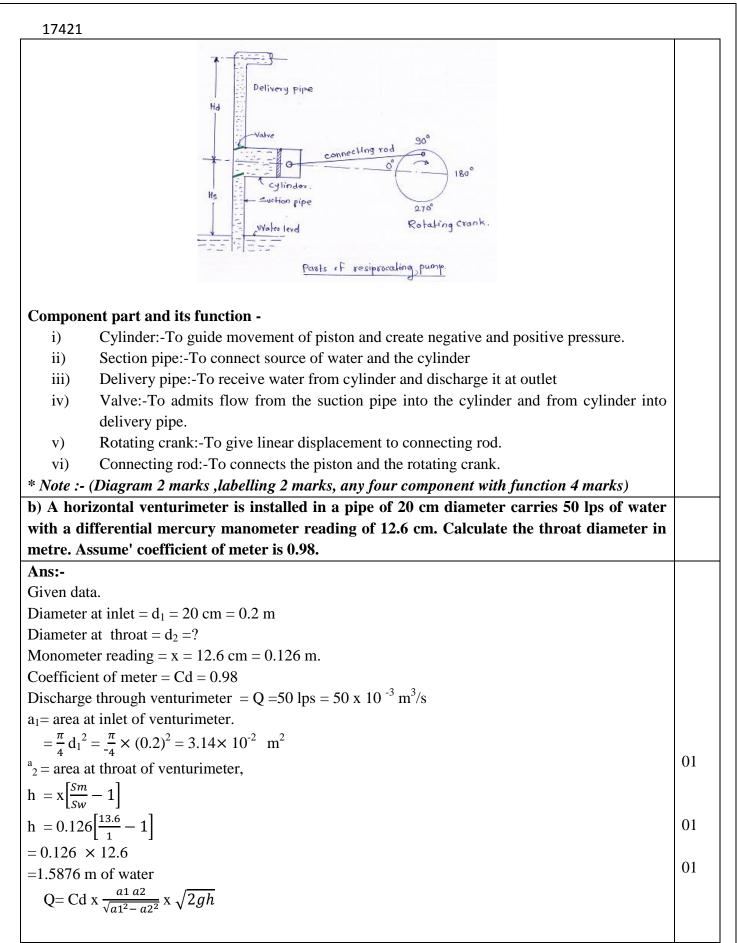
Types of flow with Froude's number (Fe) number:-

- 1) Critical flow, Fe = 1
- 2) Subcritical flow or streaming flow, Fe < 1
- 3) Super Critical or Shooing flow, Fe > 1

*(Note :-Instead of types of hydraulic jump consider types of flow)

c) A 150 mm x 75 mm venturimeter placed vertically with the throat 22.5 mm above the inlet conveys oils of specific gravity 0.78 at 29 litres per sec. Calculate the difference of pressure between inlet and throat Take Cd = 0.96.

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Ans:-	
Given data.	
Diameter at inlet = $d_1 = 150 \text{ mm} = 0.150 \text{ m}$	
Diameter at throat = $d_2 = 75 \text{ mm} = 0.075 \text{ m}$	
Coefficient of meter = $Cd = 0.96$	
Discharge = $Q = 29 \text{ lit/ sec} = 29 \text{ x } 10^{-3} \text{ m}^3/\text{sec}$	
a_1 = area at inlet of venturimeter.	
$=\frac{\pi}{4}d_1^2 = \frac{\pi}{4} \times (0.15)^2 = 1.76 \times 10^{-2} \text{ m}^2$	01
a_2^a = area at throat of venturimeter,	
$=\frac{\pi}{4}d_2^2 = \frac{\pi}{4} \times (0.075)^2 = 4.418 \times 10^{-3} \text{ m}^2$	
Sp gr of oil = 0.78	
Difference of elevations of the throat section and entrance section = $22.5 \text{ mm} = 0.0225 \text{ m}$	
We have continuity eqn.	
Q = A V	
Velocity of oil at entrance,	
$\mathbf{Q} = \mathbf{A}_1 \mathbf{x} \mathbf{V}_1$	01
$29 \times 10^{-3} = 1.76 \times 10^{-2} \times V_1$	
$V_1 = 1.65$ m/sec.	
Similarly	01
$\mathbf{Q} = \mathbf{A}_2 \mathbf{x} \mathbf{V}_2$	-
$29 \times 10^{-3} = 4.418 \times 10^{-3} \times V_2$	01
$V_2 = 6.564$ m/sec.	01
Applying Bernoulli's theorem for entrance and the throat section,	
$P_1/w + V_1^2/2g + Z_1 = P_2/w + V_2^2/2g + Z_2$	01
$P_1/w + (1.65)^2/2 \ge 9.81 + 0 = P_2/w + (6.564)^2/2 \ge 9.81 + 0.0225$	
$P_1/w + 0.139 + 0 = P_2/w + 2.2225$	
$(P_2/w) - (P_1/w) = 2.2225 - 0.139$	02
= 2.084 m of oil.	
= 20.84 cm of oil	
Q6. Attempt any two of the following.	16
a) Draw a neat sketch of a Reciprocating pump showing its various component parts.	
Mention function of each component.	
Ans:-	*



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$Q = Cd \times \frac{a1}{\sqrt{\left(\frac{a1}{a2}\right)^2 - 1}} \times \sqrt{2gh}$	01
50 x 10 ⁻³ = 0.98 x $\frac{3.14 \times 10^{-2} \times a2}{\sqrt{(3.14 \times 10^{-2})^2 - (a2)^2}}$ x $\sqrt{2 X 9.81 X 1.5876}$	02
$a_2 = 8.777 \times 10^{-3} m^2$	
$a_2 = \frac{\pi}{4} d_2^2$	
$8.777 \times 10^{-3} = (3.14/4) \times d_2^2$	02
d $_2$ = Diameter of throat = 0.1057 m	
c) Why triangular notches are preferred than rectangular notches? Find the discharge over	
the triangular notch of angle 60° when the head over the notch is 20 cm. Take Cd = 0.625.	
Triangular notches are preferred than rectangular notches because :-	
1) Only one reading i.e head (H) is required to be taken for measurement of rate of flow in a	01
given triangular notch.	
2) If in triangular notch the angle of the notch i.e. $\theta = 90^{\circ}$, the formula become very simple to remember.	01
3) A triangular notch gives more accurate result for low rate of flow than a rectangular notch.	01
4) The same triangular notch can measure a wide range of flows accurately.	01
Given data;-	
Type of notch = triangular	
Angle of notch = $\theta = 60^{0}$	
Head over notch = $H=20$ cm =0.20 m	
Coefficient of discharge = $Cd = 0.625$	
Discharge = Q =?	
For triangular notch discharge can be calculated by using formula.	
$Q = 8/15 C_{d.} \sqrt{2g} \tan \theta/2 x H^{5/2}$	02
$= 8/15 \times 0.625 \times \sqrt{2 \times 9.81} \times \tan(60/2) \times (0.20)^{5/2}$	
$=0.01525 \text{ m}^3/\text{sec}$	
Discharge over triangular notch = $0.01525 \text{ m}^3/\text{sec}$.	
	02