

MAHARASHTRA STATE BOARD OF TECHNICAL EDUCATION

(Autonomous)

(ISO/IEC-270001 – 2005 certified)

SUMMER-14 EXAMINATION

Subject code: 17310 Model Answer Page No: 1/20

Important Instructions to examiners:

- 1) The answer should be examined by keywords and not as word-to-word as given in the model answer scheme.
- 2) The model answer and the answer written by candidate may vary but the examiner may try to assess the understanding level of the candidate.
- 3) The language error such as grammatical, spelling errors should not be given more importance.(Not applicable for subject English and communication skill).
- 4) While assessing figures, examiner may give credit for principal components indicated in the figure. The figure drawn by candidate and model answer may vary. The examiner may give credit for any equivalent figure drawn.
- 5) Credits may be given step wise for numerical problems. In the some cases, the assumed constants values may vary and there may be some difference in the candidates answer and model answer.
- 6) In case of some questions credit may be given by judgment on part of examiner of relevant answer based on candidates understanding.

Q1) a).Attempt any SIX of the following	12
i) State the Primary classification of survey	
Ans.: Surveys are classified as plane survey and geodetic survey	
Plane Survey: It is the type of survey in which the mean surface of the earth is	1
considered as a plane that is curvature of the earth surface is neglected.	
Geodetic Survey: It's the type of survey in which the shape of the earth surface	1
is taken into account that is curvature of the earth surface is consider	
ii) State different objective of survey.	
Ans.: Objective of Surveys are as follows	
1) Preparation of map or Plan.	1

17310	
2) Object of survey is to determine the precise position on the surface of the	1
earth.	
iii) Define ranging and list the instrument required for ranging.	
Ranging: - "The operation of establishing intermediate points on a straight line	1
between the terminal Stations (end points) is known as ranging."	
Instrument required for ranging	
1) Ranging Rods.	1
2) Line ranger or thedolite	
iv) Write the bearing of line AB and line CD.	
B TRANCE OF THE PARTY OF THE PA	
1) D 21945	
Ans.: 1) Bearing of line AB 285 ⁰ 30'	1
2) Bearing of line CD 219 ⁰ 45'	1
v) Write any four personnel errors in compass survey.	
Ans.: 1) Inaccurate leveling of the compass box.	(1/2
2) Inaccurate centering of the compass over the stations.	mark
3) Inaccurate bisection of the object.	each
4) Taking wrong reading and recording wrongly.	any
5) Reading the Bearing in wrong direction.	four)
vi) List the accessories required for plane table survey.	
Ans.: Following are the accessories required for plane table survey.	(1/2
1) Plumbing / U- Fork with plumb bob	mark
2) Plane Alidade or telescopic Alidade	each)
3) Set of Level tube.	
4) Trough compass	
vii) Define datum line and Bench Mark.	
Ans.: <u>Datum line</u> : "It can be defined as the line parallel to the mean spheriodal earth	1
Surface from which the vertical distances are measured".	
Bench Mark: "It can be defined as the point of known elevation"	1
	ı

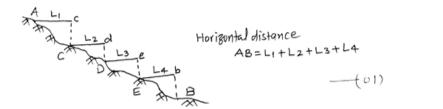
0 viii) Describe in brief negative staff reading.	
Ans.: In leveling work some points are situated below the line of sight and some	2
points are situated above the line of sight. The reduced levels of points below the	2
line of sight can be determined by taking the reading on the staff normal, where	
as when the points are situated above the line of sight the staff is required to be	
held inverted for taking the readings. The Reading taken in this position known	
as negative staff reading.	
b) Attempt any two of the following	8
i) Explain the principals of surveying.	
Ans.: Following are the principles of surveying	
1) To work from the whole to the part.	0
2) To fix the position of new station by at least two independent processes (i.e	0
the processes may be both linear, both angular, one linear & one angular.	
According to the first principal the whole area is first enclosed by main	
station and main survey line. The area is the divided in to the no. of well	0
condoned (Equilateral triangles). The propose of this is to prevent accumulation	
of errors. But if the process is reverse from part to the whole then minor errors	
in the measurement will be magnified and becomes uncontrolled	
Part area. Whole area.	
According to the Second principal the new station should be fixed by at least	
two measurements from fixed reference point. This two measurements may be	
both liner and both angular or one linear and one angular.	
A Second binciple of survey	0

 State the uses of surveying. It is used for measurement of areas. It is used for engineering commercial scientific military and navigation proposes. It is used for making plans for legal documents. 	1
2) It is used for engineering commercial scientific military and navigation proposes.	
	1
2) It is used for making plans for local decomposes	1
3) It is used for making plans for legal documents.	1
4) Accurate map of wide areas can be obtained by using surveys.	1
iii) Explain linear measurement by pacing and by speedometer.	
Ans.: Linear measurement by Pacing: This method is used only for knowing	
approximate distanced between the objects. In preliminary survey this method can	
be used for linear measurement. This method consists of counting the number of	
paces between the two points of a line. The length of the line can then be computed	2
by knowing the average length of the pace. The length of the pace varies with	
individuals, and also with the nature of the ground and the speed of pacing.	
Linear measurement by speedometer: If the nature of the ground is the	
smooth than speedometer of an automobile can be used to measure the distance	2
approximately. It works on the principals that the no of revolutions registered by the	
wheel and multiplied by the circumference of the wheel to get the distance.	
	16
Q.2 Attempt any four of the following.	10

Direct ranging	Indirect ranging	
It is done when terminal	It is done when terminal stations are	1
stations are visible	not indivisible	
It is suitable when the	It is adopted when distance is more and if obstacle	1
distance is less	like hill is intervening between the stations	1
Intermediate points are located	Intermediate two points are located approximately	
on the survey line by line ranger	in between terminal stations.	1
Fig. PEG C Direct Konging D	MI P2 I P2 I M3 I P2 I M3 M P M P M P M P M P M P M P	1

b) Expla	ain the process of chaining of	on sloping ground by	steping metghod with n	eat
sketch	h.			

Direct Method By Stepping:-



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By stepping: It consist in measuring the distance in small horizontal lengths (e.g. L_1) Let the horizontal distance between A and B be measured by a chain or tape. A portion of chain say 2m, 4m, 5m, and 10 m is stretched horizontally with one end resting on the ground and other held in line at a convenient height say 1 to 1.5 m. The point vertically below this end is then accurately marked with the help of plumb bob. The next step will then start from this point and method is continued in the correct line with point B.

c) A 30 m Chain was found to be 5 cm to short after chaining 1200 m. it was found to be 10 cm too short after chaining 2100 m. If the chain was correct before commencement of work find the true distance.

Part I: Chain was correct before commencement of work.

Length of chain L: 30 m, Error in chain 5 cm = 0.05 m too short.

Measured distanced $\mathbf{D}' = 1200 \text{ m}$

L¹=Incorrect length of chain =
$$30.00 - \left(\frac{0.05 + 0.00}{2}\right) = 29.975 \text{ m}.$$

D1 =(True distance)= Correct distance = $\left(\frac{L1}{L}\right)$ D'

= $\left(\frac{29.975}{30.00}\right)$ 1200 = 1199 m.

Part II:

Length of chain L: 30 m

Measured distanced D' =
$$(2100-1200) = 900$$

Error in chain =
$$\left(\frac{0.05 + 0.10}{2}\right) = 0.075$$

L¹=Incorrect length of chain =
$$30.00 - \left(\frac{0.05 + 0.10}{2}\right) = 29.925 \text{ m.}$$

$$D2 = (\text{True distance}) = \text{Correct distance} = \left(\frac{\text{L1}}{\text{L}}\right) \text{D'}$$

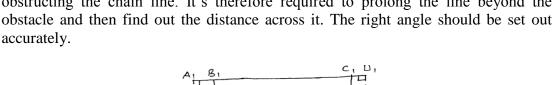
$$= \left(\frac{29.925}{30.00}\right) 900 = 897.75 \text{ m.}$$

Total True distance = 1199.00 + 897.75

Ans.:	Following points to be considered while selecting survey stations.	
1)	Main survey station should be mutually visible.	
2)	All triangles should be well conditioned.	1
3)	The survey line should lie over the leveled ground.	ma
4)	The no of survey line should be as few as practicable.	eac
5)	Each triangle should be provided at least one check line.	An
6)	The principle of survey i.e. To work from whole to the part should be strictly observed.	fou
7)	Base line should run through the center of the area so as to form the frame work of triangles on it to cover maximum area.	
e) Dra	w the sketch of chain triangulation and label the different lines.	
	ROAD	
	ROAD ROAD TIG LINE SHIPE TO ALAH TO ALAH	0

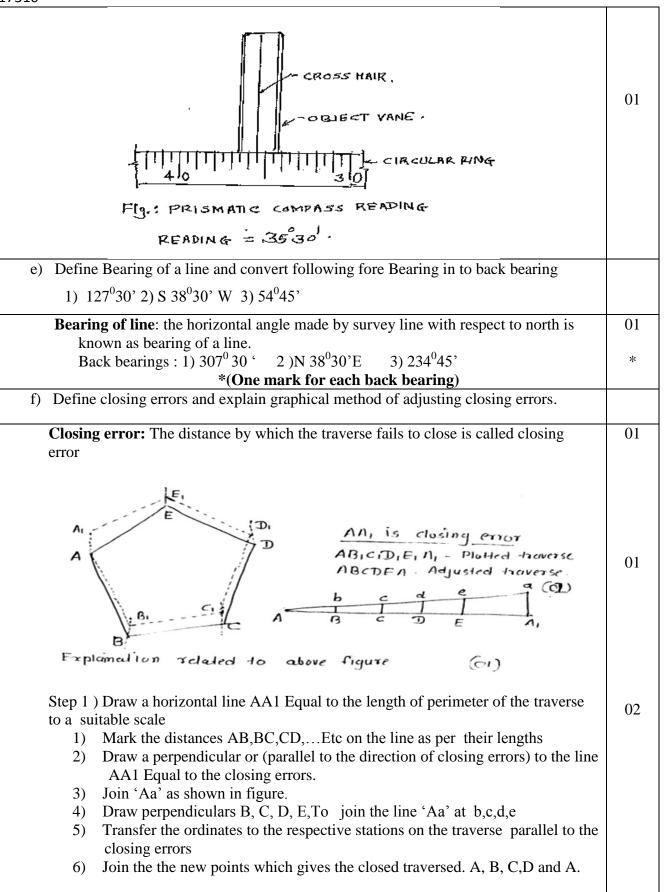
accurately.

17310 f) Explain the principal of optical square with neat sketch. **Principal of optical square:** the angle between the first incident ray and the last reflected ray is twice the angle between the two mirrors 02 If there are two plane mirrors whose reflecting surfaces make a given angle with each other and if a ray of light in plane perpendiculars to the plains of both mirrors is reflected successively from both, it undergoes a deviation of twice the angle between the reflecting surfaces. In the case of optical square the angle between the mirrors is 45° while that between the first incident ray and last reflected ray is 90° . INDEX SIGHT 02 HORIZON (a) Q.3 Attempt any four of the following 16 a) Write the obstacles in chaining and explain how you overcome when building comes across the chain line. **Obstacles in chaining** During chaining operations various operation such as River, Hill, Building, and Woods etc. are met with. All such obstacles are classified in to following categories. 1) Vision free chaining obstructed. 2) Chaining free vision obstruction. 02 3) Both Chaining and vision obstructed. 1) Both Chaining and vision obstructed: Typical example of this case is building obstructing the chain line. It's therefore required to prolong the line beyond the



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True meridian	Magnetic meridian	
The line joining the true north and the	The direction indicated by freely	
south pole is called the true meridian.	suspended and properly balanced	
	Magnetic needle unaffected by local	
	attractive force is called magnetic	ma
	meridian.	
In geodetic survey true meridian is	In Plane survey magnetic meridian is	ea
considered	considered.	(a
True meridian remains constants at all	Magnetic meridian varies from place	tw
places	to place	
Local attraction doesn't affects the true	Magnetic meridian is affected by	
meridian	local attraction	
	anced before magnetization it will not remain e magnetic influence of the earth. But it will	0
	ole. This inclination of the needle with the	U
horizontal is known as deep of the needle		
<u>-</u>	onstants but it varies from place to place in	
<u> •</u>	needle is deflected downwards and southern	
hemisphere south end of the needle is de		
c) Why zero is marked at south end on a p		
· · · · · · · · · · · · · · · · · · ·	nded on the pivot it must shows north, the	
reading under prism should be zero.	procite the sight wars, the south and will be	0
	pposite the sight vane, the south end will be	
under the prism.	he placed at the and of the people	
Obliviously the zero of marked must lead the bearing is thus obtained from source.		C
The bearing is thus obtained from soc	diff end in clock wise difection.	
d) Draw a labeled sketch of prismatic com	nass and give example of reading taken on	
prismatic compass.	pass and give example of reading taken on	
prismatic compass.		
	*,	
Te-Object	- Feye slit	
Valle	Glass Bye	
4 4 F	Valle	
Lifting Pin	- Tricm	
-GLASS COVEY	Trism	:
	Agate Prism cap	:
-GLASS COVEY	Agate Prism cap	:
-GLASS COVEY	magte of trism	:
Mirror Glass cover Graduated circle	Agate Prism Cap Focussing Stud	;
Mirror Glass cover Graduated circle	Agate Prism cap	3



Q.4 Attempt any FOUR of the following:		16
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a) Calculate interior angle in a closed traversed PQRST By following observational

Line	PQ	QR	RS	ST	TP
F. B.	S 37 ⁰ 30' E	S 43 ⁰ 15' W	N 74 ⁰ 0' W	N 11 ⁰ 00' E	N 57 ⁰ 45' E

Bearing

Line	Fore Bearing	Fore Bearing	Back Bearing
	(R.B.)	(W.C.B.)	(W.C.B.)
PQ	S 37 ⁰ 30' E	142 ° 30 '	322 0 30 '
QR	S 43 ⁰ 15' W	223 0 15	43 0 15
RS	N 74 ⁰ 0' W	286 °00 '	106 00 '
ST	N 11 ⁰ 00' E	11 000	191 °00 '
TP	N 57 ⁰ 45' E	57 ° 45 '	237 0 45 '

Calculations:

Included angle = Fore bearing of next line -B.B of previous line

= Difference (if less than 180^0 then = interior angle

(if greater than 180^0 than = Exterior angle)

And interior angle = 360° – Exterior angle

Included angle $P = 237^{\circ} 45' - 142^{\circ} 30' = 95^{\circ} 15'$

Included angle $Q = 322^{\circ} 30' - 223^{\circ} 15' = 99^{\circ} 15'$

Included angle R = $(286^{\circ}00^{\circ}43^{\circ}15^{\circ}) = 242^{\circ}45^{\circ} > 180^{\circ}$ (i.e. exterior angle)

Interior angle =
$$(360^{\circ} 00' - \text{Exterior angle})$$

= $(360^{\circ} 00' - 242^{\circ} 45')$
= $117^{\circ} 15'$

Included angle $S = 106^{\circ} 00' - 11^{\circ} 00' = 95^{\circ} 00'$

Included angle $T = 191^{\circ} 00' - 57^{\circ} 45' = 133^{\circ} 15'$

$$Total~(< P + < Q + < R + < S + < T) = 540~^0~00~^{'}$$

*(one mark each any four correct included angle)

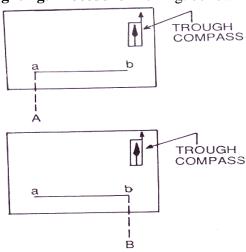
b) State different accessories of plane table survey and their use.	
i) The plane table: It is used for fixing a drawing sheet over it. To locate	
the position of the objects on the sheet by drawing rays and plotting to suitable	
scale.	*
ii) The Alidade (plain Alidade, Telescopic Alidade): It is used to sight the	
objects. To draw rays along the fiducial edge	
iii) The spirit level: It is used for leveling the plane table	
iv) The compass: (Trough Compass, Circular compass): It is used for marking	
the North direction on the map. To orient the plane table	
v) U-Fork or Plumbing fork with plumb bob: It is used for centering the	
table over the station.	
* (Any four 1/2mark for name of accessories and ½ for use.)	
c) Write any two advantages and disadvantages of plane table survey.	
Ans.: Advantages:	01
1 Comparison of plotted work and actual work is possible in the field, if required can be corrected easily.	mar
No possibility of omission of the necessary measurement.	(an
3 Its most rapid method	two
4 Field book is not necessary	two
5 Less costlier than thedolite survey6 No grate skill is required to prepare a satisfactory map	
o Two grate skill is required to prepare a satisfactory map	
Disadvantages:	01
1) It is not suitable for work in wet climate	mar
2) Several accessories are required to be carried hence possibility lost.3) Repotting of survey work to different scale is difficult in absence of field notes.	(an
4) It is heavy and difficult to carry.	
D) Fredrig (managing modes) of plant (able moments)	two
D) Explain traversing method of plane table surveying.	
D C	01

Procedure: Let plot the traverse ABCD

- 1) Set up the plane table at station A.
- 2) Select the suitable point a on the sheet.
- 3) Carry out all temporary adjustment i.e. Centering, Levelling, and Mark north direction.
- 4) With Alidade Touching at point a Bisect Station B, Draw the ray, Measure distance AB and plot point b on the ray AB with suitable scale
- 5) Shift the table to the station B, Carry out all temporary adjustment and orientation by back sighting clamp the table.
- 6) Similarly repeat the above steps at successive station at C, D,.
- 7) At the end traverse A, B, C, D is obtained.
- 8) Checks lines may run to check the traverse.
- 9) If any error is then may be adjusted.
- 10) It is as shown in fig.
- E) Explain with neat sketch method of orientation by back sighting of plane table survey

Orientation: The method of setting up the plane table at each of the successive stations parallel to the position it occupied at the starting station is known as orientation.

Orientation by back sighting: Procedure- Ref Fig. bellow



i) Suppose A and B are two stations. The plane table is set up over A. the table is leveled by spirit level and centered by U-fork so that point 'a' is just over station A. The north line is marked on the right hand top corner of the sheet by trough compass.

ii) With the alidade touching 'a', the ranging rod at B is bisected and a ray is drawn. The distance AB is measured and plotted to any suitable scale. So the point 'b' represents station B.

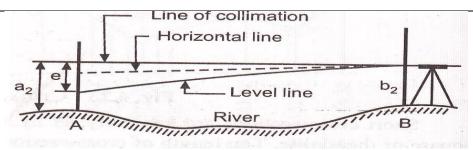
iii) The table is shifted and set up over B. It is leveled and centered so that 'b' just over B. Now the alidade is placed along the line 'ba', and the ranging rod at A is bisected by turning the table clockwise or anticlockwise. When the centering, leveling and bisection of the ranging rod at A are perfect, then the orientation is said to be perfect.

03

02

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F) Define line of collimation and axis of bubble tube.	
Line of collimation : The line joining the intersection of cross hairs on diaphragm to the optical center of object glass and its continuation up to the object is known as line of collimation.	02
Axis of bubble tube: It is an imaginary line tangential to the longitudinal curve of the tube at its middle point.	02
5) Attempt any FOUR of the following.	16
a) Describe the advantages of Auto level.	
 Following are the advantages of auto level: 1) Operational Comfort: Use of Auto level is free of fatigue, does not cause strain on eyes, nerves etc. 2) High precision: Precision of auto level is about ± 0.5mm to 0.8 mm for one km 	*
 3) High speed Time required for leveling is about half of that required with ordinary level. 4) Freedom from errors: Bubble usually remain in center and image of staff is erect, easy to read 	
5) Freedom from external influences: No influence of cloud, rain, magnetic field, vibrations, etc. on the leveling work with Auto level.6) Range of application: Auto can be used for medium and large size project. To establish bench marks accurately.	
* Any four one mark each.	
b) Explain with neat sketch reciprocal leveling	
Reciprocal leveling is adopted i) to find the difference in level accurately Between the two points considerably apart. ii) When it is not possible to set up the level in between the two points due to river or pond etc. In this error due to curvature and refraction, collimation errors, error in instrument adjustment are eliminated.	01
Procedure: Let A and B be the two points on opposite banks of the river. Let it is required to find out the level difference between A and B Step 1) Case I:Set up the level very near to A and with bubble in centre take the readings on the staff held at A and B Let these reading are aland b1 Line of collimation Horizontal line	*
2) Case II: Shift the level and set up very near to B and with bubble in center take the readings on the staff held at A and B, these reading are a2 and b 3)	



3) Let d=true difference of level between A and B

Let e= Total error (error due to curvature and refraction , imperfect adjustment)

In Case I, The correct reading on B = b1-e

In case II, the correct reading on A = a2-e

From A, the true difference of level between A and B, d = (b1-e)-a1

Or d = (b1-a1)-e equation 1

From B, the true difference of level between A and B, d= b2-(a2-e)

= (b2-a2)+e equation 2

To eliminate total error e adding equation 1 and 2

2d = (b1-a1) - e + (b2-a2) + e

d = ((b1-a1) + (b2-a2))/2

The error is eliminated and the true difference is equal to the mean of the two apparent differences of level between A and B

*(Procedure 02 mark and sketch 01 mark)

c) Distinguish between the following

i) Back sight and Foresight

Back sight	Fore sight		
It is a staff reading taken on a point of known elevation i.e. B.M. or C.P.	It is a staff reading taken on a point whose elevation is to be determined. i.e. C.P		01
It is the first riding taken after the level is set up and levelled.	It is the last staff reading denoting the shifting of the instrument or closing of leveling work.	1	01

ii) Simple leveling and differen	ntial leveling	
Simple leveling	Differential leveling	
In this difference in elevation between two point is determined, when two points are visible, small distance apart, small difference in elevation between two points.	In this difference in elevation between two point is determined if the points are too far apart, elevation between two point is more or any obstacle in between them.,	01
MA O B	A C, O, C, O, B	01

10	
d) Define fly leveling and explain the situation when fly leveling is required.	
<u>Fly leveling:</u> The process of leveling to determine the elevation of the points which	
are some distance apart by usually taking only B.S. and F.S. without any	02
distance measurement is known as fly leveling. It is form of differential leveling.	
Situations for adopting fly level:	
1) To connect a bench mark to the starting point of the alignment of any project.	
2) to check the accuracy of the leveling work by connecting B.M. to any	02
intermediate point of the alignment	
STARTING POINT OF C ALIGNMENT B CP = CHANGE POINT L, L_2= POSITION OF LEVEL	
e) Enlist the sources of errors in leveling and explain any one source in detail.	
2) Emist the sources of citors in leveling and explain any one source in detail.	
The sources of error in levelling are as bellow	
1) Instrumental errors: Error due to imperfect adjustment, Sluggish bubble, Faulty	01
Focusing tube, Erroneous leveling staff.	
2) Personal errors: Errors of manipulation, Imperfect sighting ,Errors due to settlement	
of staff and level stand	
3) Errors due to natural sources	
1) Instrumental errors:	
i) Error due to imperfect adjustment: Line of sight is inclined upward or downwards.	
Axis of the bubble tube not being perpendicular to the vertical axis, line of collimation	
not being parallel to the bubble axis. Instrument should be get tested and adjusted	
before use, equalize the fore sight and back sight distance to minimize this error.	
ii)Sluggish bubble: when bubble is sluggish then it does not show correct horizontal	
Line of sight. To get good work replace it by good bubble tube.	
iii)Faulty focusing tube: Due to this object does not move in horizontal plane during	
focusing	
iv)Erroneous leveling staff: Division on the leveling staff are faulty. Replace it by	
correct staff.	
2) Personal Errors:	
i) Errors of manipulation: Careless leveling, Bubble not in center, Resting hands on	
tripod, Staff not being held exactly vertical.	*
ii) Imperfect sighting: Poor focusing, Parallax error not removed,	
iii)Error due to settlement of staff and level: set up the instrument, and hold the staff	
on firm ground, proper selection of C.P. and instrument station location.	
3) Errors due to natural causes:	
i) Curvature of earth ii)Refraction iii)Effect of wind iv) effect of sun	
, , , , , , , , , , , , , , , , , , ,	
In ordinary leveling the curvature and refraction error are not consider as they are very	
In ordinary leveling the curvature and refraction error are not consider as they are very less 0.003m for 300m length. Use umbrella to shelter the instrument, use sun shade,	
In ordinary leveling the curvature and refraction error are not consider as they are very	

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f) Explain the precautionary measures in leveling	
Precautionary measures in leveling work are as follow	
1) Do not take sighting distance too large (less than 100m) depending upon	
range of instrument	
2) Held the staff exactly vertical	01
3) Do the adjustment of instrument properly and carry out tests of instruments	mark
4) See that bubble must be in center always	(any
5) Leveling must be carried out in favorable conditions only.	four)
6) Balance the B.S. and F.S. distances to avoid the errors.	,
7) Use good quality and correct leveling staff.	
8) Check the leveling work at the end of every day.	
9) Read the staff correctly	
Q6) Attempt any TWO of the following:	16
a) i) Find the area of the plot ABCD from the data collected in the chain and cross staff	10
survey (Refer Figure No.1)	
survey (Refer Figure 140.1)	
7109	
70- D80	
Service 1	
58119	
C60	
8 (0	
0 400	
16-70	
10 A 20	
1 🕭 0 1	
100	
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70	
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58.60	
30C	
(5) Including 4, we	
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(I)	
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10 - A 20	
The state of the second of the	
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act - cv	1

Fig.	Fig.	chai	nage	Base	off	set	Mean		(m2)	Net
No.	,	From	TO	590	I	I	Offset	+	1 -	Aream
1	V	16.70	30	13.30	0	40	20	266	l.	
2	521	30	50	20	40	60	50	1000		-
3		50	58.60	8.60	60	0	30	258	× 11	1001
4	1	58.60	70	11.40	0	80	40		456	4001
5	[7	10	70	60	20	80	50	3000	67	
6	5	10	1670	6.70	20	0	10	OWN	67	3
						7:0	Total	4524	523	

Ans:- Area of the plot ABCD = 4001 m²
(Note also consider area calculation of each fig. separately)

*(½ mark for each area and ½ mark for total Area)

ii) Q and R are two points on the opposite banks of a river along a chain line PQR which crosses the river at right angles to the bank. From a point A which is 96.2m from Q along the bank, the bearing of R is 3050 30' and the bearing of P is 2150 30'. If the length of PQ is 150m. Find the width of river.

fiver width, 3263 Using Principle

9620m A of Similar Triangle

15000m BOS30 QA QP Q.A.

$$\frac{Ag}{gR} = \frac{Pg}{gA}.$$
Given $\Rightarrow Pg = 150.0m$, $AQ = 96.20m$.

3. $AQR = Width \Rightarrow f River$.

$$\frac{96.20}{QR} = \frac{150.0}{96.20}$$

$$\Rightarrow QR = 61.69 \text{ m}. Ans. = Width of River}.$$

02

01

Q.6. b) Following bearings were observed in running a closed traverse PQRST with prismatic compass Calculate the included angle and find out corrected F.B. and B.B. with usual check

2.2		
Line	F.B.	B.B.
PQ	80 ⁰ 10'	259 ⁰ 0'
QR	120° 20°	301 ⁰ 50'
RS	170 ⁰ 50'	350 ⁰ 50'
ST	230 ⁰ 10'	49 ⁰ 30'
TP	310 ⁰ 20'	130 ⁰ 15'

Tabular Result

				-	1		
Line	Obs. F.B.	Obs.	Obs. Dick-Angle	corr- ection	Correct -ed Angle	Correct, F.B.	and the same of th
199	800	25900	2P=5005'	05'	50 10	80 401	260 40
9R	120 20	30150	LQ=138 40	05'	138 45	121 50	301 55
RS	170° 50	350 50	LR=131°00	05'	131 05	170 50	350°501
ST	238 ાઇ	49°36	LS=120° 40	05'	120 45	230 05	50°05
TP	310 20	136 15	LT= 99° 10'	051	99' 15"	310° 50'	130 50'
		SUM	539 35	25 1	540 001	01 + 1	

Calculations-D Included angles-LP = 130 15 - 80 10 = 5005' 02 19 = 259° 40'- 120° 20'= 138°40' LR = 301°50'- 170'50'= 131°00' L5 = 350°50' - 230°10'= 120° 40' closed Traxerse. LT = 310° 20' - 49° 30' = 99° 10' PARST . observed, SUM = 539°351. Theoretical sum of all included = (2n-4)80 = (2x5-4)90= 54000. observed sum = 538 35 , therefore correction is required assuming equal precautions at all station, equal correction at each station, 25/5 = 05, corrected angle as above. II] corrected bearing. Consider the line Rs; FB, and BB. diff. = 180; B-B- of Resline Rs = 350°501 02 F.B. of line ST = 230° 051 T.B. of line ST. = 230° 05' B.B. of dine ST 50° 05 - 99 15 + 3600 F.B. of line T.P 310° 50' -180000 BB of line T.P. 130 50 50° 10' 80° 40 02 180 00 BB- of line Pg 260 40 138 45 F.B. of line GR 121°55 + 180 00 BB of lone OR 301° 55 131 05' kine RS 170°50 + 180 00 350'50' = Obs. B.B. of line R.S. 02 O.K. Check is varified All calculated values are shaon in table

17310

c) The following page of old level book having few staff reading missing find out the missing reading and rewrite the page. Apply usual checks.

Stn		Staff reading		H.I.	R.L.	Remark
	BS	IS	FS			
1	2.650				100.000	B.M.
2		X			98.910	
3		3.830			98.820	
4	4.640				98.380	CP1
5		0.380				
6	1.640			103.700	102.060	CP2
7		2.840			100.860	
8		3.480		104.900	100.220	CP3
9					102.700	End Stn

Solution:

Stn	Stn Staff i		ng	H.I.	R.L.	Remark
	BS	IS	FS			
1	2.650			102.650	100.000	B.M.
2		3.740			98.910	
3		3.830			98.820	
4	4.640		4.270	103.020	98.380	CP1
5		0.380			102.640	
6	1.640		0.960	103.700	102.060	CP2
7		2.840			100.860	
8	4.680		3.480	104.900	100.220	CP3
9			2.200		102.700	End Stn
	$\Sigma = 13.610$		$\Sigma = 10.910$			

Calculation:

1)100.000 + 2.650 = 102.650

5) 103.020 - 0.380 = 102.640

2)102.650 -98.910 =3.740

6)103.020 -102.060 =0.960

3) 102.650 -98.380 =4.270

7)104.900 -100.220 =4.680

4) 98.38 +4.640 =103.020

8)104.900 -102.700 =2.200

Check Σ BS – Σ FS = Last R.L. – First R.L 13.610 - 10.910 = 102.700 -100.000 2.700 = 2.700

O.K Check is verified.

*(correct missing reading 04 marks rewriting the page of field book 2 mark,02 marks for check)